Presentation Day



"Integrated Design and Structural Analysis of a Multi-Storied Residential Building in Pink City, Raozan"

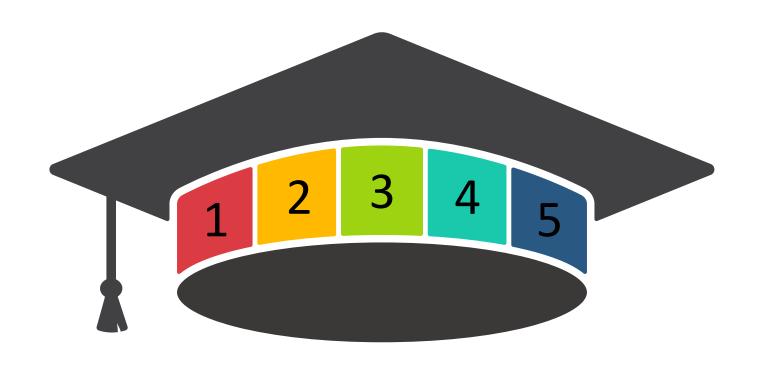


Course Code – CE 412

Team - Equilibrium

ID- 1901042-49

Objective of our **Project**









Code Compliance



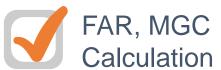
Sustainable Materials



Cost Efficiency

Previous work









Masterplan Collection



Study BNBC 2020





Revise Design



Serviceability Check



Bearing Capacity Calculation

Recent work









Masterplan Collection



Study BNBC







Revise Design



Serviceability Check

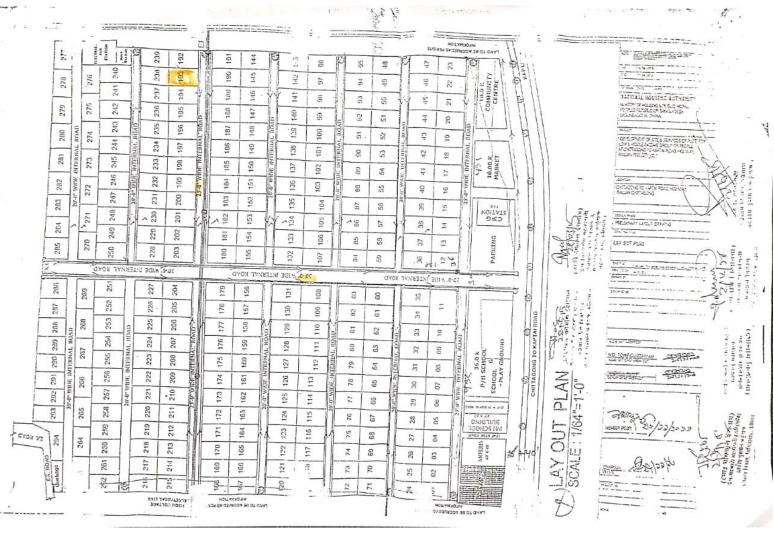


Bearing Capacity Calculation

Site Visit

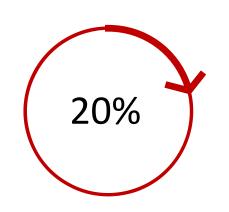
Masterplan



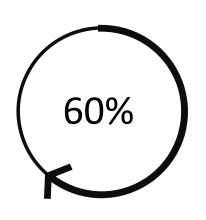


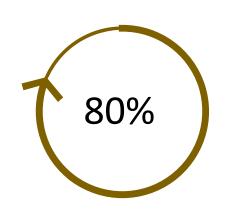
FAR Analysis

Remaining Work











Reinforcement Check

Overstress Check

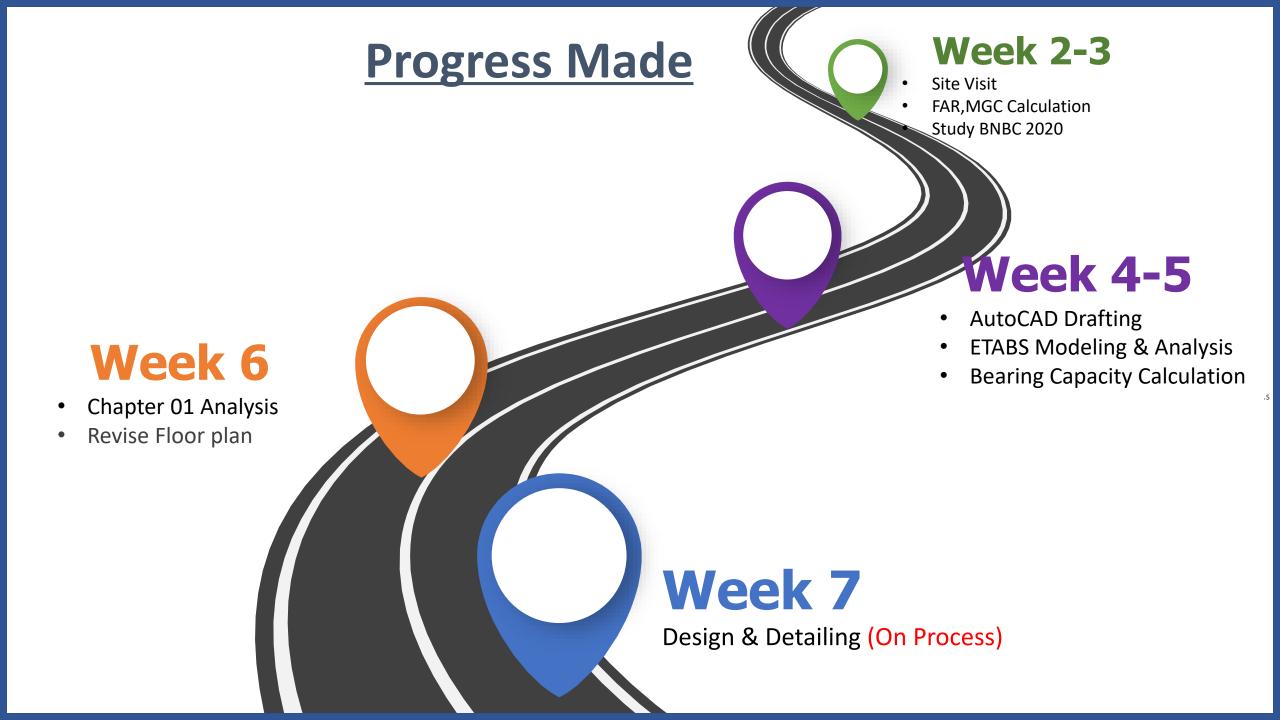
Detailing

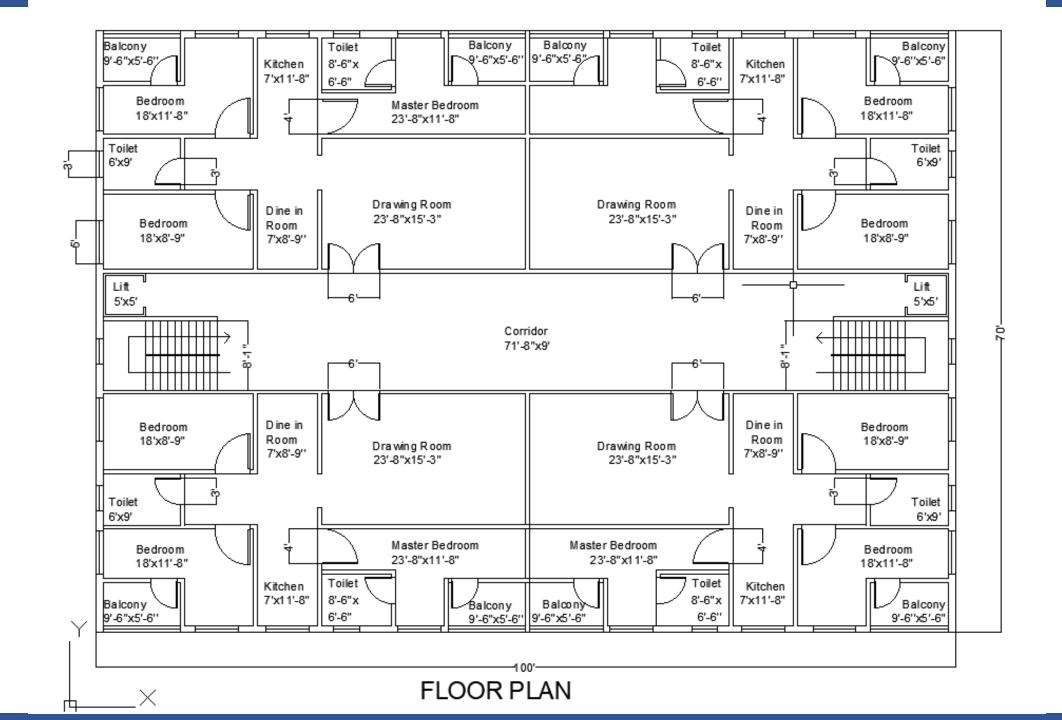
- **Extra Design**
- Cost Benefit Ratio

- Column Rebar
 - Beam Rebar
 - Shear wall

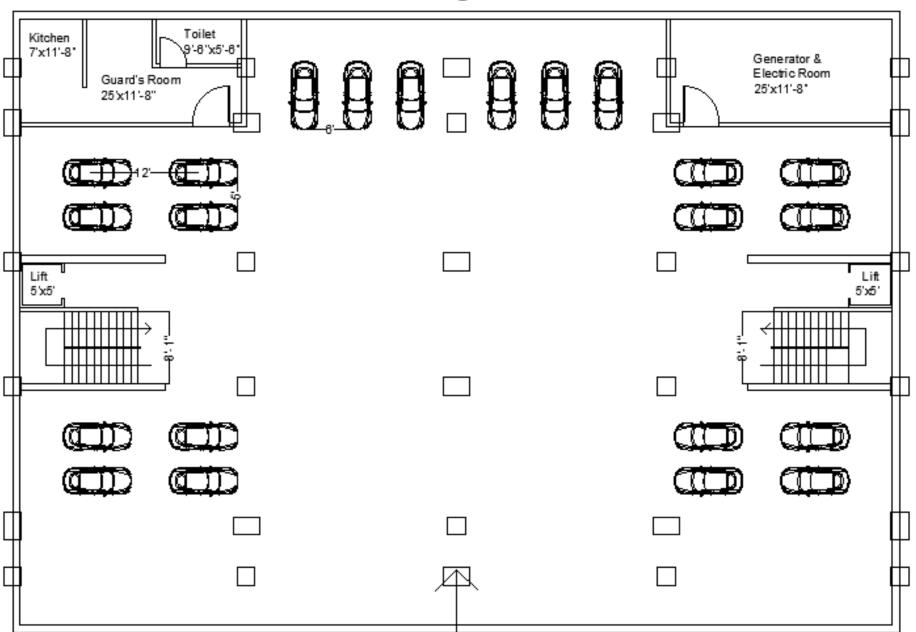
- Column Rebar
- Beam Rebar
- Foundation

- Overhead Tank
- Pipe network
- Electricity line

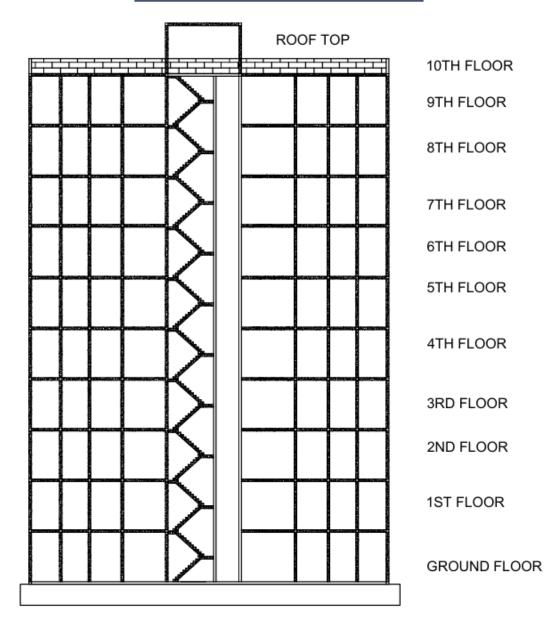




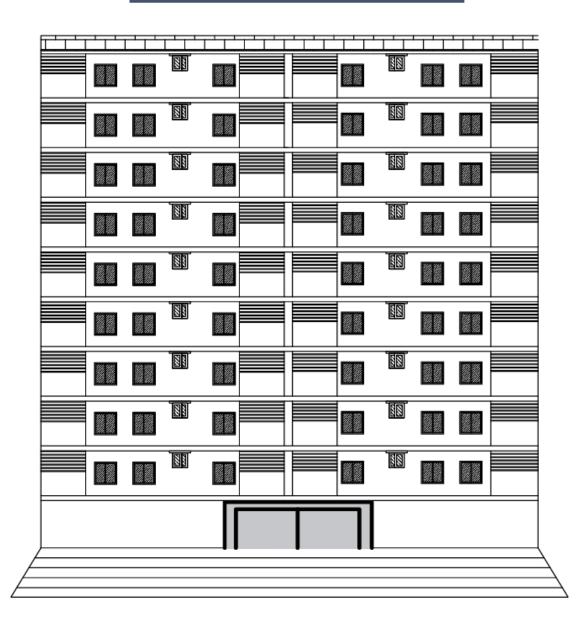
Parking Plan



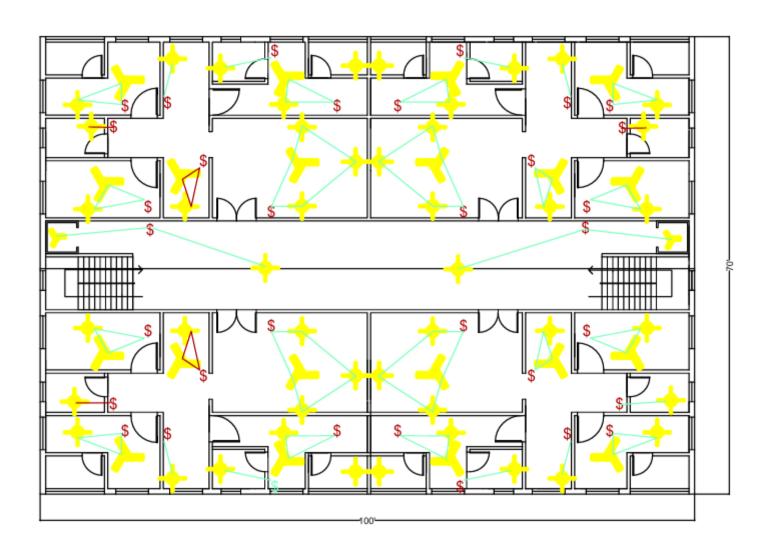
Cross section



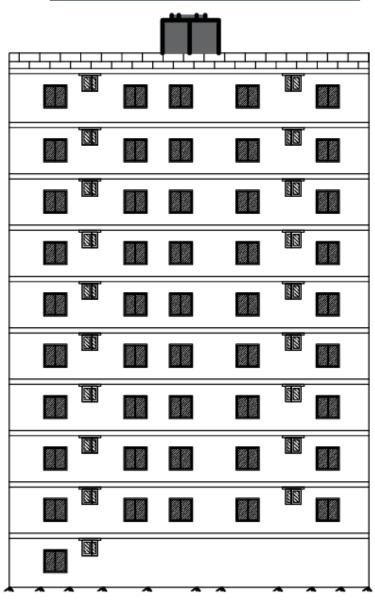
Front Elevation



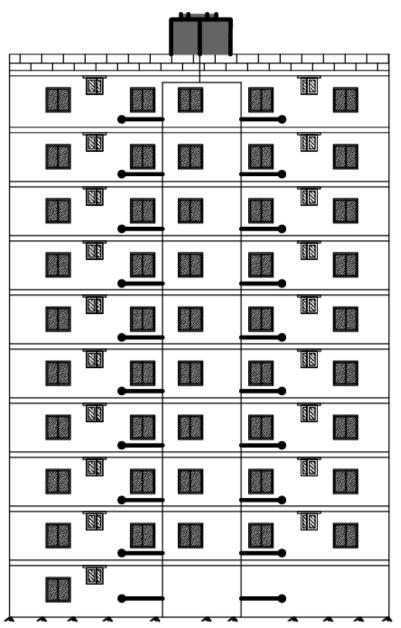
Electrical Plan



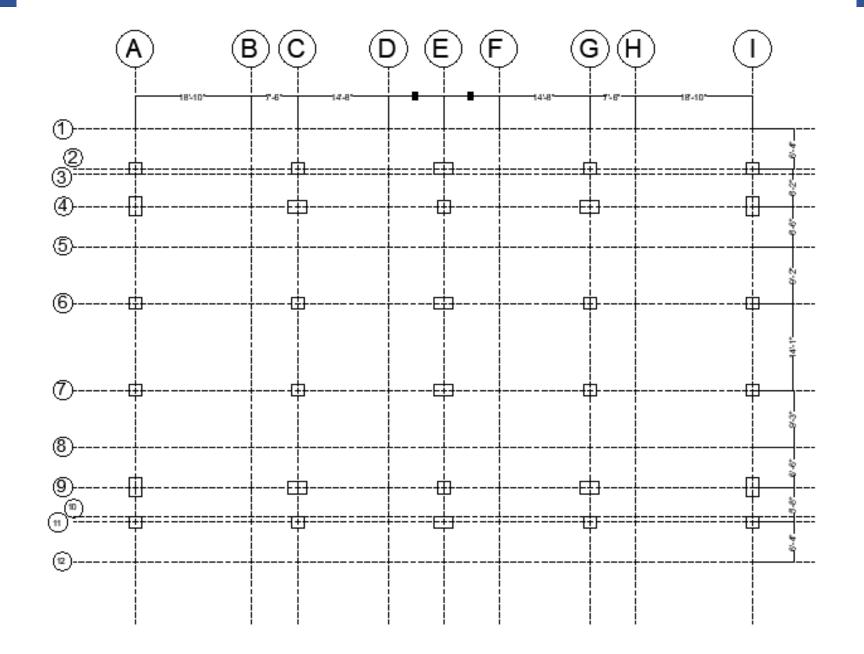
Side Elevation



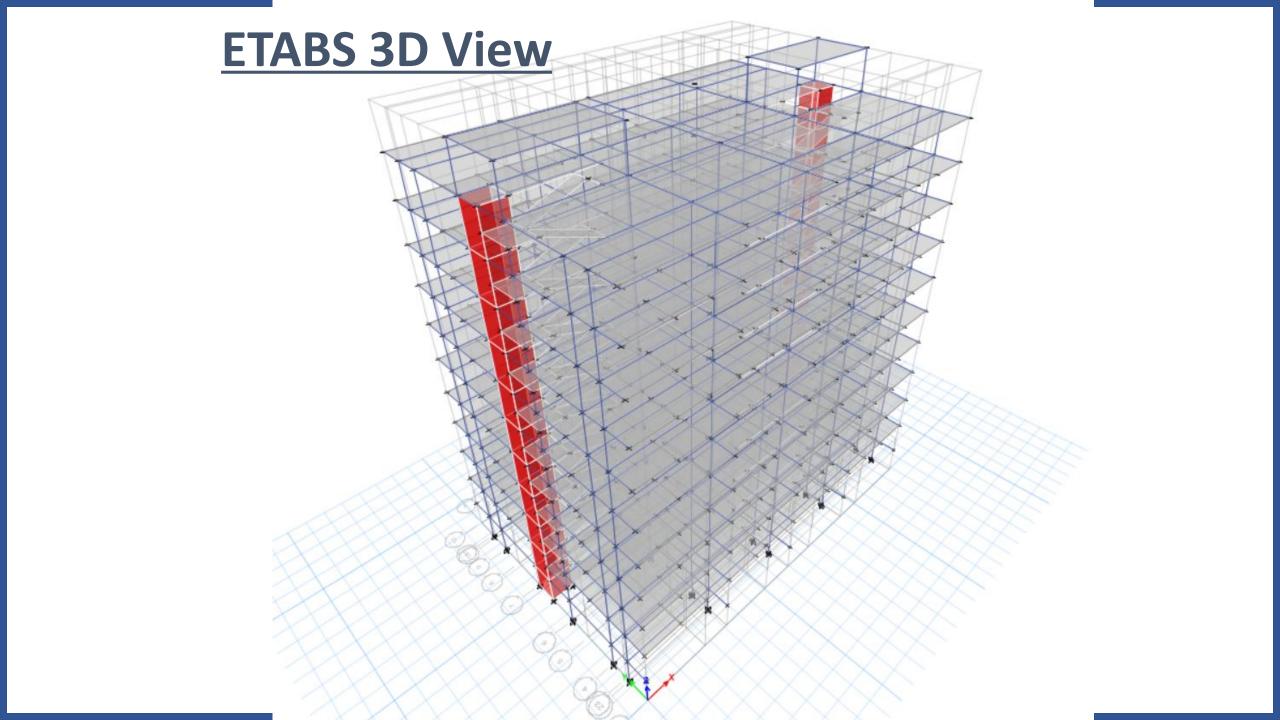
Plumbing Layout



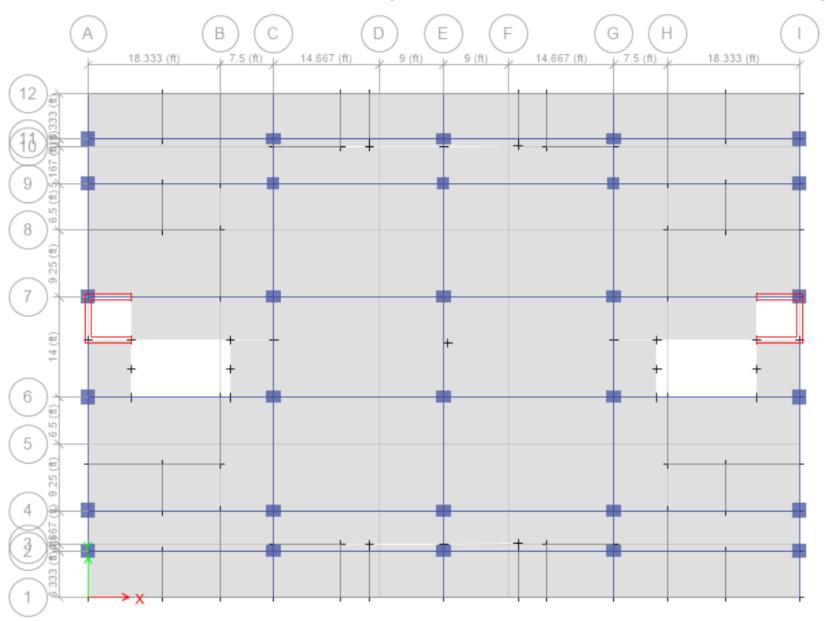
<u>Plan</u>



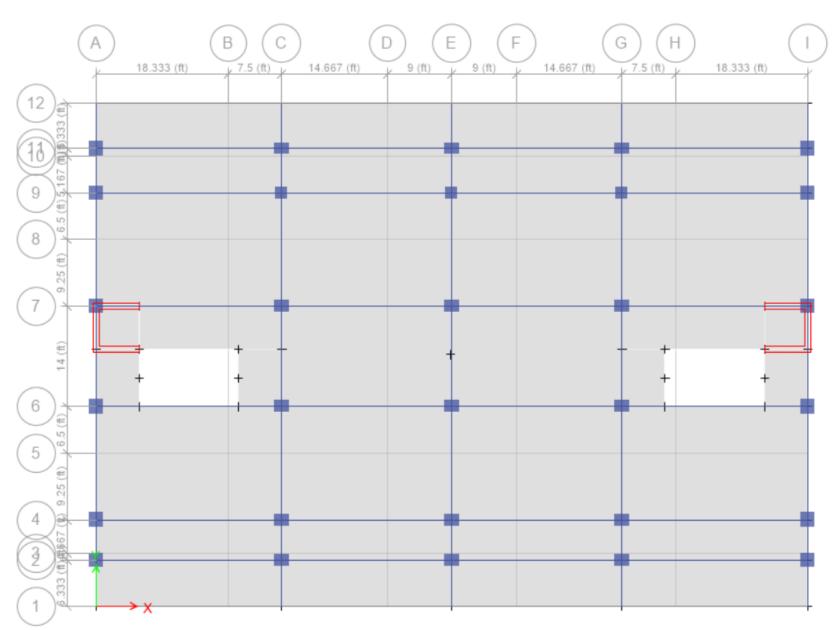
	-26'-4"	22'-8"	22'-8"	26'-4"	1
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ETABS Plan View (from 1st to 9th floor)



ETABS Plan View (10th floor)



2.2.4 Weight of Materials and Constructions

Load assign

In estimating dead loads, the actual weights of materials and constructions shall be used, provided that in the absence of definite information, the weights given in Tables 6.2.1 and 6.2.2 shall be assumed for the purposes of design.

Table 6.2.1: Unit Weight of Basic Materials

Material	Unit Weight (kN/m³)	Material	Unit Weigh (kN/m³)	
Aluminium	27.0	Granite, Basalt	26.4	
Asphalt	21.2	Iron - cast	70.7	
Brass	83.6	- wrought	75.4	
Bronze	87.7	Lead	111.0	
Brick	18.9	Limestone	24.5	
Cement	14.7	Marble	26.4	
Coal, loose	8.8	Sand, dry	15.7	

Table 6.2.2: Weight of Construction Materials.

Material/Component/Member	Veight per Unit Area (kN/m²)	Material/Component/Member	Weight per Unit Area (kN/m²)
Floor		Walls and Partitions	
Asphalt, 25 mm thick	0.526	Acrylic resin sheet, flat, per	0.012
Clay tiling, 13 mm thick	0.268	mm thickness	
Concrete slab (stone aggregate)*:		Asbestos cement sheeting:	
solid, 100 mm thick	2.360	4.5 mm thick	0.072
solid, 150 mm thick	3.540	6.0 mm thick	0.106
Galvanized steel floor deck (excl.	0.147-	Brick masonry work, excl.	
topping)	0.383	plaster:	
Magnesium oxychloride:		burnt clay, per 100 mm	1.910
normal (sawdust filler), 25 mm	0.345	thickness	
thick	: S-2009200	sand-lime, per 100 mm	1.980
heavy duty (mineral filler),	0.527	thickness	
25 mm thick	0.555.7.3)	Concrete (stone aggregate)*:	
Terrazzo paving 16 mm thick	0.431	100 mm thick	2.360

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Material/Component/Member	Veight per Unit Area (kN/m²)	Material/Component/Member	Weight pe Unit Area (kN/m²)
Roof		150 mm thick	3.540
Acrylic resin sheet, corrugated:		250 mm thick	5.900
3 mm thick, standard corrugations 3 mm thick, deep corrugations	0.043	Fibre insulation board, per 10 mm thickness	0.034
Aluminium, corrugated sheeting: (incl. lap and fastenings)	0.002	Fibrous plaster board, per 10 mm thickness	0.092
1.2 mm thick	0.048	Glass, per 10 mm thickness	0.269
0.8 mm thick	0.048	Hardboard, per 10 mm	0.961
0.6 mm thick	0.028	thickness	
Aluminium sheet(plain):	2337230	Particle or flake board, per 10 mm thickness	0.075
1.2 mm thick	0.033	Plaster board, per 10 mm	0.092
1.0 mm thick	0.024	thickness	
0.8 mm thick	0.019	Plywood, per 10 mm	0.061
Bituminous felt (5 ply) and gravel	0.431	thickness	
Slates:	100000000	Ceiling	
4.7 mm thick	0.335	Fibrous plaster, 10 mm thick	0.081
9.5 mm thick	0.671	Cement plaster, 13 mm thick	0.287
Steel sheet, flat galvanized:	0000000	Suspended metal lath and	0.480
1.00 mm thick	0.082	plaster	
0.80 mm thick	0.067	(two faced incl. studding)	
0.60 mm thick	0.053	Miscellaneous	
Steel, galvanized std. corrugated sheeting:		Felt (insulating), per 10 mm thickness	0.019
(incl. lap and fastenings)		Plaster:	
1.0 mm thick	0.120	Cement plaster, per 10	0.230
0.8 mm thick	0.096	mm thickness	
0.6 mm thick Tiles :	0.077	Lime plaster, per 10 mm thickness	0.191
terra-cotta tiles (French pattern)	0.575	PVC sheet, per 10 mm	0.153
concrete , 25 mm thick	0.527	thickness	
clay tiles	0.6-0.9	Rubber paving, per 10 mm thickness	0.151
		Terra-cotta Hollow Block Masonry:	
		75 mm thick	0.671
		100 mm thick	0.995
		150 mm thick	1.388

^{*} For brick aggregate, 90% of the listed values may be used.

Occupancy or Use	Uniform kN/m ²	Concentrated kN	
Hotels	See Residential		
Libraries			
Reading rooms	2.90	4.50	
Stack rooms	7.20 d	4.50	
Corridors above first floor	3.80	4.50	
Manufacturing*		30	
Light	4.00	6.00	
Medium	6.00	9.00	
Heavy	12.00	13.40	
Garments manufacturing floor except stacking or storage area	4.00°		
Stacking or storage area of garments manufacturing industry	6.00 /	10.00 /	
Marquees	3.60	,	
Office Buildings			
File and computer rooms shall be designed for heavier loads based on anticipated occupancy			
Lobbies and first-floor corridors	4 80	9 00	
Offices	2.40	9.00	
Corridors above first floor	3.80	9.00	
Penal Institutions	20000	3	
Cell blocks	2.00		
Corridors	4.80		
Residential		SA)	
Dwellings (one- and two-family)			
Uninhabitable attics without storage	0.50	177	
Uninhabitable attics with storage	1.00		
Habitable attics and sleeping areas	1.50	107	
All other areas except stairs and balconies	2.00	1944	
Hotels and multifamily houses	1 - 100		
Private rooms and corridors serving them	2.00	1922	
Public rooms and corridors serving them	4.80	127	
Reviewing stands, grandstands, and bleachers	4.80 g	1522	

Occupancy or Use	Uniform kN/m ²	Concentrated kN	
Roofs			
Ordinary flat roof	1.00 h		
Pitched and curved roofs	See Table 6.2.4		
Roofs used for promenade purposes	2.90	223	
Roofs used for roof gardens or assembly purposes	4.80	44.5	
Roofs used for other special purposes	See N	ote ' below	
Awnings and canopies			
Fabric construction supported by a lightweight rigid skeleton structure	0.24 (nonredu- ceable)	####	
All other construction	1.00	==0)	
Primary roof members exposed to a work floor			
Single panel point of lower chord of roof trusses or any point along primary structural members supporting roofs over manufacturing, storage warehouses, and repair garages	722	9.00	
All other occupancies	15	1.33	
All roof surfaces subject to maintenance workers		1.33	
Schools		:	
Classrooms	2.00	4.50	
Corridors above first floor	3.80	4.50	
First-floor corridors	4.80	4.50	
Scuttles, skylight ribs, and accessible ceilings		0.90	
Sidewalks, vehicular driveways, and yards subject to trucking	12.00 ^j	35.60 ^k	
Stadiums and arenas			
Bleachers	4.80 ^g	**	
Fixed seats (fastened to floor)	2.90 ^g	2433	
Stairs and exit ways One- and two-family residences only	4.80 2.00	See Note/ below	

Table 6.2.20: Values for Coefficients to Estimate Approximate Period

Structure type	C_t	m	
Concrete moment-resisting frames	0.0466		Note: Consider moment resisting frames as frames
Steel moment-resisting frames	0.0724	0.8	which resist 100% of seismic force and are not
Eccentrically braced steel frame	0.0731		enclosed or adjoined by components that are more
All other structural systems	0.0488	0.75	rigid and will prevent the frames from deflecting under seismic forces.

Earthquake load assign

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Site	Description of soil	Average Soi	l Properties in top	30 meters
Class	meters depth	Shear wave velocity, \overline{V}_s (m/s)	SPT Value, N (blows/30cm)	Undrained shear strength, \overline{S}_u (kPa)
sc	Deep deposits of dense or medium dense sand, gravel or stiff clay with thickness from several tens to many hundreds of metres.	180 - 360	15 - 50	70 - 250
SD	Deposits of loose-to- medium cohesionless soil (with or without some soft cohesive layers), or of predominantly soft-to- firm cohesive soil.	< 180	< 15	< 70
SE	A soil profile consisting of a surface alluvium layer with V_s values of type SC or SD and thickness varying between about 5 m and 20 m, underlain by stiffer material with $V_s > 800$ m/s.	1855	5758	50.
S ₁	Deposits consisting, or containing a layer at least 10 m thick, of soft clays/silts with a high plasticity index (PI > 40) and high water content	< 100 (indicative)		10 - 20
S ₂	Deposits of liquefiable soils, of sensitive clays, or any other soil profile not included in types SA to SE or S1	125		22

Table 6.2.15: Seismic Zone Coefficient Z for Some Important Towns of Bangladesh

Town	Z	Town	Z	Town	Z	Town	Z
Bagerhat	0.12	Gaibandha	0.28	Magura	0.12	Patuakhali	0.12
Bandarban	0.28	Gazipur	0.20	Manikganj	0.20	Pirojpur	0.12
Barguna	0.12	Gopalganj	0.12	Maulvibazar	0.36	Rajbari	0.20
Barisal	0.12	Habiganj	0.36	Meherpur	0.12	Rajshahi	0.12
Bhola	0.12	Jaipurhat	0.20	Mongla	0.12	Rangamati	0.28
Bogra	0.28	Jamalpur	0.36	Munshiganj	0.20	Rangpur	0.28
Brahmanbaria	0.28	Jessore	0.12	Mymensingh	0.36	Satkhira	0.12
Chandpur	0.20	Jhalokati	0.12	Narail	0.12	Shariatpur	0.20
Chapainababganj	0.12	Jhenaidah	0.12	Narayanganj	0.20	Sherpur	0.36
Chittagong	0.28	Khagrachari	0.28	Narsingdi	0.28	Sirajganj	0.28
Chuadanga	0.12	Khulna	0.12	Natore	0.20	Srimangal	0.36
Comilla	0.20	Kishoreganj	0.36	Naogaon	0.20	Sunamganj	0.36
Cox's Bazar	0.28	Kurigram	0.36	Netrakona	0.36	Sylhet	0.36
Dhaka	0.20	Kushtia	0.20	Nilphamari	0.12	Tangail	0.28
Dinajpur	0.20	Lakshmipur	0.20	Noakhali	0.20	Thakurgaon	0.20
Faridpur	0.20	Lalmanirhat	0.28	Pabna	0.20	10000	
Feni	0.20	Madaripur	0.20	Panchagarh	0.20		

Table 6.2.18: Seismic Design Category of Buildings

Site	Occupa	ancy Cate	egory I, II	and III	Occupancy Category IV			
Class	Zone 1	Zone 2	Zone 3	Zone 4	Zone 1	Zone 2	Zone 3	Zone
SA	В	С	C	D	С	D	D	D
SB	В	C	D	D	С	D	D	D
SC	В	C	D	D	С	D	D	D
SD	С	D	D	D	D	D	D	D
SE, S ₁ , S ₂	D	D	D	D	D	D	D	D

Seismic Force-Resisting System	Response Reduction Factor, R	System Overstrength Factor, Ω_{θ}	Deflection Amplification Factor, C _d	Seismic Design Category B	Seismic Design Category C	Seismic Design Category D
				Не	ight limit	(m)
B.BUILDING FRAME SYSTEMS (with bracing or shear wall)		300	(A) (A)			
Steel eccentrically braced frames, moment resisting connections at columns away from links	8	2	4	NL	NL	50
2. Steel eccentrically braced frames, non-moment- resisting, connections at columns away from links	7	2	4	NL	NL	50
3. Special steel concentrically braced frames	6	2	5	NL	NL	50
4. Ordinary steel concentrically braced frames	3.25	2	3.25	NL	NL	11
5. Special reinforced concrete shear walls	6	2.5	5	NL	NL	50
6. Ordinary reinforced concrete shear walls	5	2.5	4.25	NL	NL	NP
7. Ordinary reinforced masonry shear walls	2	2.5	2	NL	50	NP
8. Ordinary plain masonry shear walls	1.5	2.5	1.25	18	NP	NP
C. MOMENT RESISTING FRAME SYSTEMS (no shear wall)	363	315	92	š		
Special steel moment frames	8	3	5.5	NL	NL	NL
2. Intermediate steel moment frames	4.5	3	4	NL	NL	35
3. Ordinary steel moment	3.5	3	3	NL	NL	NP

Table 6.2.16: Site Dependent Soil Factor and Other Parameters Defining Elastic Response Spectrum

Soil type	S	$T_B(s)$	$T_C(s)$	$T_D(s)$
SA	1.0	0.15	0.40	2.0
SB	1.2	0.15	0.50	2.0
sc	1.15	0.20	0.60	2.0
SD	1.35	0.20	0.80	2.0
SE	1.4	0.15	0.50	2.0

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PARTVI

Appendix C
Seismic Design Parameters for Alternative Method of Base Shear Calculation

Table 6.C.1: Spectral Response Acceleration Parameter S₅ and S₁ for Different Seismic Zone

Parameters	Zone-1	Zone-2	Zone-3	Zone-4
Ss	0.3	0.5	0.7	0.9
S ₁	0.12	0.2	0.28	0.36

Table 6.C.2: Site Coefficient Fa for Different Seismic Zone and Soil Type

Soil Type	Zone-1	Zone-2	Zone-3	Zone-4
SA	1.0	1.0	1.0	1.0
SB 1.2		1.2	1.2	1.2
SC	1.15 1.15 1		1.15	1.15
SD 1.35		1.35	1.35	1.35
SE 1.4		1.4	1.4	1.4

Table 6.C.3: Site Coefficient F_{ν} for Different Seismic Zone and Soil Type

Soil Type	Zone-1	Zone-2	Zone-3	Zone-4	
SA	1.0	1.0	1.0	1.0	
SB 1.5		1.5	1.5	1.5	
SC	1.725 1.725		1.725	1.725	
SD 2.7		2.7	2.7	2.7	
SE 1.75		1.75	1.75	1.75	

PARTVI Appendix C Seismic Design Parameters for Alternative Method of Base Shear Calculation

Table 6.C.1: Spectral Response Acceleration Parameter S_S and S₁ for Different Seismic Zone

Parameters	Zone-1	Zone-2	Zone-3	Zone-4	
Ss	0.3	0.5	0.7	0.9	
S ₁	0.12	0.2	0.28	0.36	

Table 6.C.2: Site Coefficient Fa for Different Seismic Zone and Soil Type

Soil Type	Zone-1	Zone-2	Zone-3	Zone-4
SA	1.0	1.0	1.0	1.0
SB 1.2		1.2 1.2		1.2
SC	1.15	1.15 1.15 1		1.15
SD 1.35		1.35	1.35	1.35
SE	1.4	1.4	1.4	1.4

Table 6.C.3: Site Coefficient F_{ν} for Different Seismic Zone and Soil Type

Soil Type	Zone-1	Zone-2	Zone-3	Zone-4	
SA	1.0	1.0	1.0	1.0	
SB 1.5		1.5	1.5	1.5	
SC	1.725 1.72		1.725	1.725	
SD 2.7		2.7	2.7	2.7	
SE 1.75		1.75	1.75	1.75	

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Table 6.2.16: Site Dependent Soil Factor and Other Parameters Defining Elastic Response Spectrum

Soil type	S	$T_B(s)$	$T_C(s)$	$T_D(s)$
SA	1.0	0.15	0.40	2.0
SB	1.2	0.15	0.50	2.0
sc	1.15	0.20	0.60	2.0
SD	1.35	0.20	0.80	2.0
SE	1.4	0.15	0.50	2.0

Wind load assign

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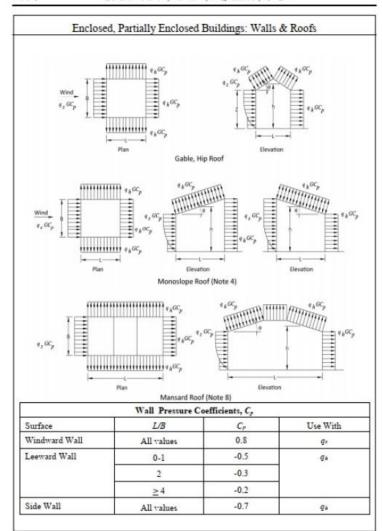


Figure 6.2.6 External Pressure Coefficients, C_P main wind force resisting system -Method 2 (All Heights)

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Table 6.2.8: Basic Wind Speeds, V, for Selected Locations in Bangladesh

Location	Basic Wind Speed (m/s)	Location	Basic Wind Speed (m/s
Angarpota	47.8	Lalmonirhat	63.7
Bagerhat	77.5	Madaripur	68.1
Bandarban	62.5	Magura	65.0
Barguna	80.0	Manikganj	58.2
Barisal	78.7	Meherpur	58.2
Bhola	69.5	Maheshkhali	80.0
Bogra	61.9	Moulvibazar	53.0
Brahmanbaria	56.7	Munshiganj	57.1
Chandpur	50.6	Mymensingh	67.4
Chapai Nawabgani	41.4	Naogaon	55.2
Chittagong	80.0	Narail	68.6
Chuadanga	61.9	Narayanganj	61.1
Comilla	61.4	Narsinghdi	59.7
Cox's Bazar	\$0.0	Natore	61.9
Dahagram	47.8	Netrokona	65.6
Dhaka	65.7	Nilphamari	44.7
Dinajpur	41.4	Noakhali	57.1
Faridpur	63.1	Pabna	63.1
Feni	64.1	Panchagarh	41.4
Gaibandha	65.6	Patuakhali	80.0
Gazipur	66.5	Pirojpur	80.0
Gopalgani	74.5	Rajbari	59.1
Habigani	54.2	Rajshahi	49.2
Hatiya	80.0	Rangamati	56.7
Ishurdi	69.5	Rangpur	65.3
Joypurhat	56.7	Satkhira	57.6
Jamalpur	56.7	Shariatpur	61.9
Jessore	64.1	Sherpur	62.5
Jhalakati	80.0	Sirajganj	50.6
Jhenaidah	65.0	Srimangal	50.6
Khagrachhari	56.7	St. Martin's Island	80.0
Khulna	73.3	Sunamganj	61.1
Kutubdia	80.0	Sylhet	61.1
Kishoreganj	64.7	Sandwip	80.0

2.4.6 Exposure

For each wind direction considered, the upwind exposure category shall be based on ground surface roughness that is determined from natural topography, vegetation, and constructed facilities.

2.4.6.1 Wind directions and sectors

For each selected wind direction at which the wind loads are to be evaluated, the exposure of the building or structure shall be determined for the two upwind sectors extending 45° either side of the selected wind direction.

The exposures in these two sectors shall be determined in accordance with Sections 2.4.6.2 and 2.4.6.3 and the exposure resulting in the highest wind loads shall be used to represent the winds from that direction.

2.4.6.2 Surface roughness categories

A ground surface roughness within each 45° sector shall be determined for a distance upwind of the site as defined in Sec 2.4.6.3 from the categories defined in the following text, for the purpose of assigning an exposure category as defined in Sec 2.4.6.3.

Surface Roughness A: Urban and suburban areas, wooded areas, or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

Surface Roughness B: Open terrain with scattered obstructions having heights generally less than 9.1 m. This category includes flat open country, grasslands, and all water surfaces in cyclone prone regions.

Surface Roughness C: Flat, unobstructed areas and water surfaces outside cyclone prone regions. This category includes smooth mud flats and salt flats.

2.4.6.3 Exposure categories

Exposure A: Exposure A shall apply where the ground surface roughness condition, as defined by Surface Roughness A, prevails in the upwind direction for a distance of at least 792 m or 20 times the height of the building, whichever is greater.

Exception: For buildings whose mean roof height is less than or equal to 9.1 m, the

Table 6.2.9: Importance Factor, I (Wind Loads)

Occupancy Category ¹ or Importance Class	Non-Cyclone Prone Regions and Cyclone Prone Regions with V = 38-44 m/s	Cyclone Prone Regions with V > 44 m/s
I	0.87	0.77
п	1.0	1.00
III	1.15	1.15
IV	1.15	1.15

¹ The building and structure classification categories are listed in Table 6.1.1

2.4.7.2 Topographic factor

The wind speed-up effect shall be included in the calculation of design wind loads by using the factor K_{zt} :

$$K_{zt} = (1 + K_1 K_2 K_3)^2 (6.2.5)$$

Where, K_1 , K_2 , and K_3 are given in Figure 6.2.4. If site conditions and locations of structures do not meet all the conditions specified in Sec 2.4.7.1 then $K_{zt} = 1.0$.

2.4.8 Gust Effect Factor

2.4.8.1 Rigid structures

For rigid structures as defined in Sec 2.1.3, the gust-effect factor shall be taken as 0.85 or calculated by the formula:

$$G = 0.925 \frac{1+1.7g_Q I_2 Q}{1+1.7g_Q I_2}$$
(6.2.6)

$$G = 0.925 \frac{1+1.7g_Q I_2 Q}{1+1.7g_v I_2}$$

$$I_z = c \left(\frac{10}{\bar{z}}\right)^{1/6}$$
(6.2.6)

Table 6.2.20: Values for Coefficients to Estimate Approximate Period

Structure type	C_t	m	
Concrete moment-resisting frames	0.0466	0.9	Note: Consider moment
Concrete moment-resisting frames Steel moment-resisting frames Eccentrically braced steel frame	0.0724	0.8	which resist 100% of seismic force and are not
Eccentrically braced steel frame	0.0731	0.75	enclosed or adjoined by components that are more
All other etructural eveteme	0.0488	0.75	rigid and will prevent the

Drift check for Ey	in X direction	0								
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Amplified displacement (in)	Dflection Amplification factor,Cd	Importan ce factor,	Story drift (in)	Allowable drift limit	Allowable drift (in)	Result
Stair Room Roof	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story10	10	120	0	0	5	1	0.00	0.02	2.4	ОК
Story9	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story8	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story7	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story6	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story5	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story4	10	120	0	0	5	1	0.00	0.02	2.4	ОК
Story3	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story2	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story1	10	120	0	0	5	1	0.00	0.02	2.4	OK
Plinth Level	8	96	0	0	5	1	0.00	0.02	1.92	ОК
oundation Level	0	0	0	0	5	1	0.00	0.02	0	OK

	P delta	for Ey										
Story	Height (ft)	Height (in)	Gravity force/Stor y force (P)	Story shear (Vy)	Displace ment	Story drift	θ	Cd	Importan ce factor	в	^θ max	Result
Stair Room Roof	10	120	184.607	-26.515	3.38112	1.146545	0.013304	5	1	1	0.1	No P delta
Story10	10	120	1801.416	-268.128	3.151811	1.1619	0.01301	5	1	1	0.1	No P delta
Story9	10	120	4151.701	-550.945	2.919431	1.33436	0.016759	5	1	1	0.1	No P delta
Story8	10	120	6501.985	-795.909	2.652559	1.48796	0.020259	5	1	1	0.1	No P delta
Story7	10	120	8852.27	-1004.44	2.354967	1.6169	0.02375	5	1	1	0.1	No P delta
Story6	10	120	11202.56	-1178.06	2.031587	1.71591	0.027195	5	1	1	0.1	No P delta
Story5	10	120	13552.84	-1318.47	1.688405	1.76971	0.030319	5	1	1	0.1	No P delta
Story4	10	120	15903.12	-1427.53	1.334463	1.762655	0.032727	5	1	1	0.1	No P delta
Story3	10	120	18253.41	-1507.37	0.981932	1.679855	0.033903	5	1	1	0.1	No P delta
Story2	10	120	20603.69	-1560.48	0.645961	1.50501	0.033119	5	1	1	0.1	No P delta
Story1	10	120	22956.46	-1590.02	0.344959	1.724795	0.041504	5	1	1	0.1	No P delta
Plinth Level	8	96	23961.23	-1594.86	0	0	0	5	1	1	0.1	No P delta
Foundation Level	0	0	0	0	0	0	0	5	1	1	0.1	No P delta

P Delta Check According to BNBC 2020 P delta for Ex Gravity θ Cd Height (ft) force/Stor Height Story Displace Story Importan ^θmax В y force (P) shear (Vx) (in) Story drift ce factor Result ment Stair Room 184.607 -26.515 2.742222 10 120 0.9717 0.011276 0.1 No P delta Roof 0.94157 0.010543 0.1 No P delta Story10 10 1801.416 -268.128 2.936562 0.1 No P delta 10 4151.701 -550.945 2.748248 1.13071 0.014201 Story9 1.31406 0.017891 0.1 No P delta Story8 10 120 6501.985 -795.909 2.522106 0.1 No P delta 10 120 8852.27 -1004.44 2.259294 1.488105 0.021858 Story7 Story6 10 11202.56 -1178.06 1.961673 1.627625 0.025796 0.1 No P delta 0.1 No P delta Story5 10 13552.84 -1318.47 1.636148 1.718085 0.029434 15903.12 0.032367 0.1 No P delta Story4 10 -1427.53 1.292531 1.74325 -1507.37 0.943881 0.1 No P delta 10 18253.41 1.673485 0.033775 Story3 0.032515 0.1 No P delta Story2 10 20603.69 -1560.48 0.609184 1.47756 10 22956.46 -1590.02 0.313672 1.56836 0.03774 0.1 No P delta Story1 0.1 No P delta Plinth Level 8 23961.23 -1594.86 Foundation 0 0 0 0.1 No P delta 0 0 0 Level

Dilit Check for	Ey in Y direction	<u>///</u>					
Story	Drift ratio	Amplified drift ratio	Dflection Amplification factor,Cd	Importan ce factor,	Allowable drift ratio	Result	
Stair Room Roof	0.001932	0.00966	5	1	0.02	OK	
Story10	0.001994	0.00997	5	1	0.02	OK	
Story9	0.002229	0.011145	5	1	0.02	OK	
Story8	0.002488	0.01244	5	1	0.02	OK	
Story7	0.002715	0.013575	5	1	0.02	OK	
Story6	0.002882	0.01441	5	1	0.02	OK	
Story5	0.002967	0.014835	5	1	0.02	OK	
Story4	0.002949	0.014745	5	1	0.02	OK	
Story3	0.002817	0.014085	5	1	0.02	OK	
Story2	0.002545	0.012725	5	1	0.02	OK	
Story1	0.002068	0.01034	5	1	0.02	OK	
Plinth Level	0.001434	0.00717	5	1	0.02	OK	
oundation Level	0	0	5	1	0.02	OK	

Drift check for Wy	in Y direction						
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Story drift	Allowable drift limit	Allowable drift	Result
Stair Room Roof	10	120	2.163801	0.252871	0.004	0.48	ОК
Story10	10	120	1.91093	0.149785	0.004	0.48	OK
Story9	10	120	1.761145	0.162272	0.004	0.48	OK
Story8	10	120	1.598873	0.174906	0.004	0.48	OK
Story7	10	120	1.423967	0.187574	0.004	0.48	ОК
Story6	10	120	1.236393	0.19869	0.004	0.48	OK
Story5	10	120	1.037703	0.206701	0.004	0.48	OK
Story4	10	120	0.831002	0.209782	0.004	0.48	OK
Story3	10	120	0.62122	0.204739	0.004	0.48	OK
Story2	10	120	0.416481	0.18951	0.004	0.48	ОК
Story1	10	120	0.226971	0.226971	0.004	0.48	OK
Plinth Level	8	96	0	0	0.004	0.384	ОК
Foundation Level	0	0	0	0	0.004	0	OK

orsional I	rregularity Chec	ck According to	BNBC 2020								
load case	1st end displacement of upper story	1st end displacement of lower story	2nd end displacement of upper story	2nd end displacement of lower story	1st end drift	2nd end drift	Maximum Drift	Average drift	Maximum Drift / Average drift	Torsional irregularity check result	Extreme torsional irregularity check result
Ex	3.040601	2.945547	1.890641	1.778645	0.095054	0.111996	0.111996	0.103525	1.081825646	REGULAR	REGULAR
Ev	3.150017	2.914758	3.145142	2.909723	0.235259	0.235419	0.235419	0.235339	1.000339935	REGULAR	REGULAR

	For wind lo	oad .					
Drift check for Wx	in X direction						
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Story drift	Allowable drift limit	Allowable drift	Result
Stair Room Roof	10	120	0.958275	0.042034	0.004	0.48	ОК
Story10	10	120	1.000309	0.059779	0.004	0.48	ОК
Story9	10	120	0.94053	0.070377	0.004	0.48	ОК
Story8	10	120	0.870153	0.081275	0.004	0.48	ОК
Story7	10	120	0.788878	0.093076	0.004	0.48	ОК
Story6	10	120	0.695802	0.10424	0.004	0.48	ОК
Story5	10	120	0.591562	0.113707	0.004	0.48	ОК
Story4	10	120	0.477855	0.12006	0.004	0.48	ОК
Story3	10	120	0.357795	0.120364	0.004	0.48	ОК
Story2	10	120	0.237431	0.111482	0.004	0.48	ОК
Story1	10	120	0.125949	0.125949	0.004	0.48	ОК
Plinth Level	8	96	0	0	0.004	0.384	ОК
oundation Level	0	0	0	0	0.004	0	OK

Drift Che	ck According	to BNBC 20	20								
<u>F</u>	or earthquak	e load									
Drift check for Ex	in X direction										
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Amplified displac	cement (in)	Dflectio Amplificat factor,0	on ce factor	42 W. Can 12 U.	Allowable drift limit	Allowable drift (in)	Result
Stair Room Roof	10	120	2.742222	13.7111	13.71111		1	0.97	0.02	2.4	OK
Story10	10	120	2.936562	14.6828	14.68281		1	0.94	0.02	2.4	OK
Story9	10	120	2.748248	13.74124		5	1	1.13	0.02	2.4	OK
Story8	10	120	2.522106	12.6105	53	5	1	1.31	0.02	2.4	OK
Story7	10	120	2.259294	11.2964	47	5	1	1.49	0.02	2.4	OK
Story6	10	120	1.961673	9.80836	55	5	1	1.63	0.02	2.4	OK
Story5	10	120	1.636148	8.1807	4	5	1	1.72	0.02	2.4	OK
Story4	10	120	1.292531	6.46265	55	5	1	1.74	0.02	2.4	OK
Story3	10	120	0.943881	4.71940	05	5	1	1.67	0.02	2.4	ОК
Story2	10	120	0.609184	3.0459	2	5	1	1.48	0.02	2.4	OK
Story1	10	120	0.313672	1.5683	6	5	1	1.57	0.02	2.4	OK
Plinth Level	8	96	0	0		5	1	0.00	0.02	1.92	ОК
Foundation Level	0	0	0	0		5	1	0.00	0.02	0	ОК

Story	Displacement Check/	Sway Limitation/Late	ral Displacement/Hor	izontal Displacement ch	<u>eck</u>
oad Combination	Maximum Story Displacement (in)	Story Height Avobe Ground (ft)	Story Height Avobe Ground (in)	Allowable Story Displacement (in)	Result
D + L + 0.7Wx	0.719742	110	1320	2.64	ОК
D + L - 0.7Wx	0.271884	110	1320	2.64	OK
D + L + 0.7Wy	1.327721	110	1320	2.64	OK
D + L - 0.7Wy	0.271555	110	1320	2.64	OK

Story Displacement Check/Sway Limitation/Lateral Displacement/Horizontal Displacement check Maximum Story Displacement Allowable Story Displacement Story Height Avobe Ground (in) Load Combination Story Height Avobe Ground (ft) Result (in) (in) 1320 110 2.64 OK D + L + 0.7Wx0.719742 110 1320 2.64 OK D+L-0.7Wx 0.271884 110 1320 2.64 OK D + L + 0.7Wy1.327721 110 1320 2.64 OK D + L - 0.7Wy 0.271555

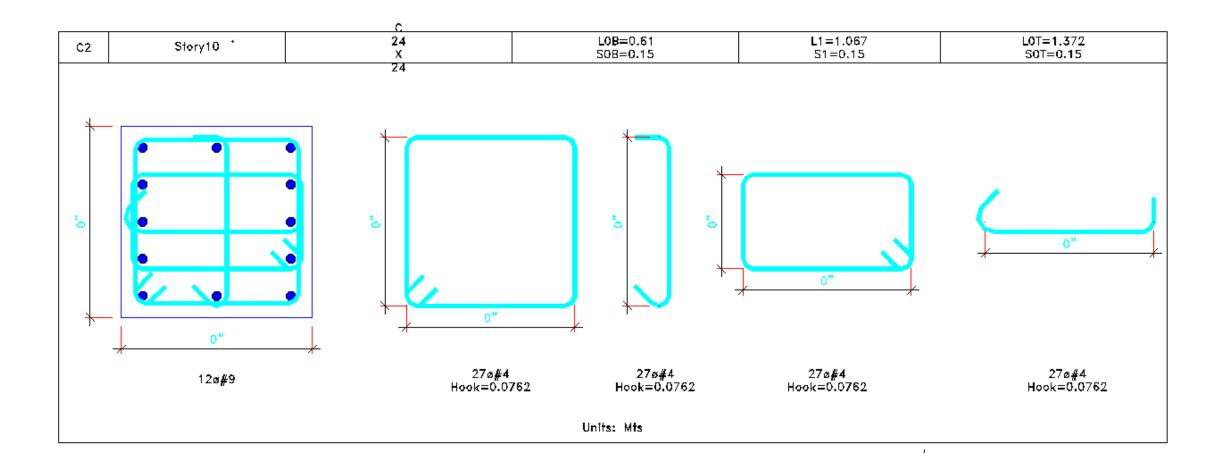
Drift check for Ey	in Y direction				8)					
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Amplified displacement (in)	Dflection Amplification factor,Cd	Importan ce factor,	Story drift (in)	Allowable drift limit	Allowable drift (in)	Result
Stair Room Roof	10	120	3.38112	16.9056	5	1	1.15	0.02	2.4	OK
Story10	10	120	3.151811	15.759055	5	1	1.16	0.02	2.4	OK
Story9	10	120	2.919431	14.597155	5	1	1.33	0.02	2.4	OK
Story8	10	120	2.652559	13.262795	5	1	1.49	0.02	2.4	OK
Story7	10	120	2.354967	11.774835	5	1	1.62	0.02	2.4	OK
Story6	10	120	2.031587	10.157935	5	1	1.72	0.02	2.4	OK
Story5	10	120	1.688405	8.442025	5	1	1.77	0.02	2.4	OK
Story4	10	120	1.334463	6.672315	5	1	1.76	0.02	2.4	OK
Story3	10	120	0.981932	4.90966	5	1	1.68	0.02	2.4	OK
Story2	10	120	0.645961	3.229805	5	1	1.51	0.02	2.4	OK
Story1	10	120	0.344959	1.724795	5	1	1.72	0.02	2.4	OK
Plinth Level	8	96	0	0	5	1	0.00	0.02	1.92	OK
Foundation Level	0	0	0	0	5	1	0.00	0.02	0	OK

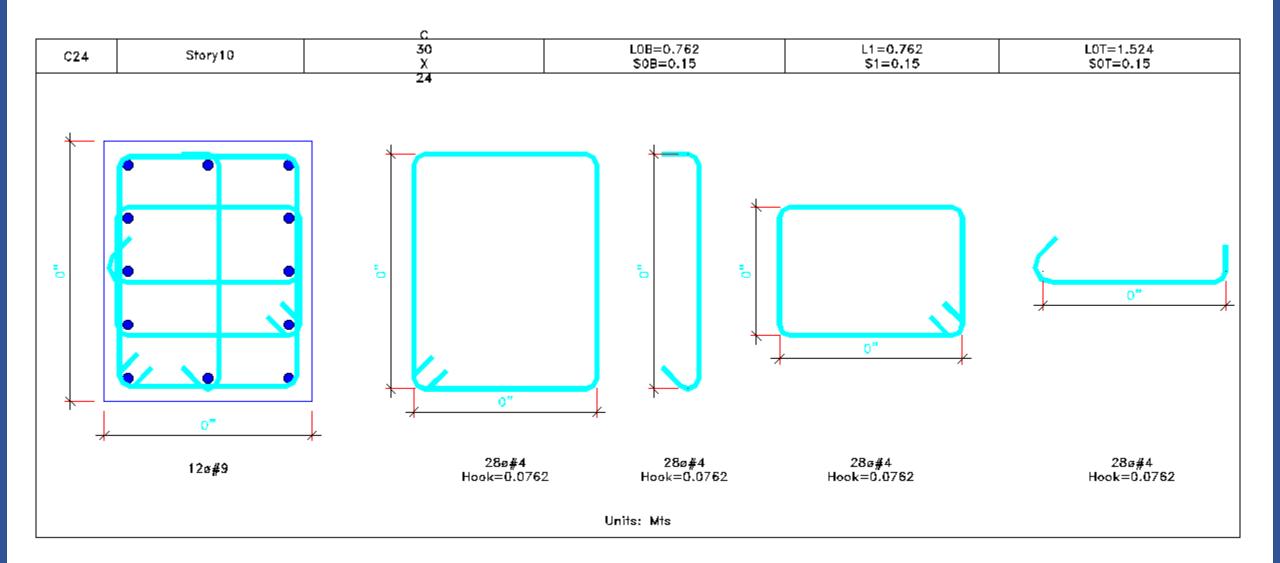
Drift check for Ex i	n Y direction									
Story	hsx(ft)	hsx (in)	Elastic displacem ent(in)	Amplified displacement (in)	Dflection Amplification factor,Cd	Importan ce factor,	Story drift (in)	Allowable drift limit	Allowable drift (in)	Result
Stair Room Roof	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story10	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story9	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story8	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story7	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story6	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story5	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story4	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story3	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story2	10	120	0	0	5	1	0.00	0.02	2.4	OK
Story1	10	120	0	0	5	1	0.00	0.02	2.4	OK
Plinth Level	8	96	0	0	5	1	0.00	0.02	1.92	OK
Foundation Level	0	0	0	0	5	1	0.00	0.02	0	OK

Column

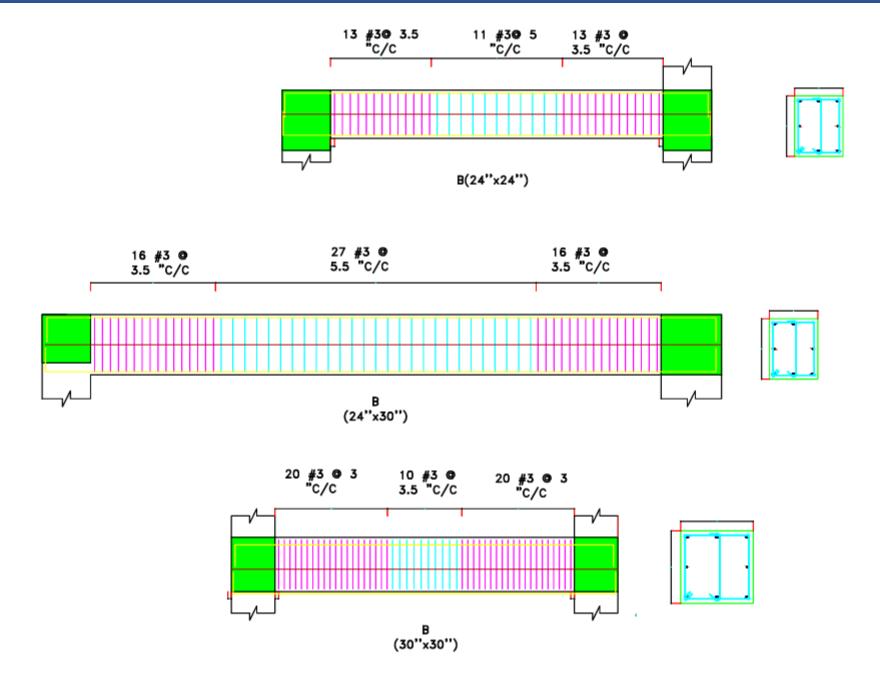
Ŷ			100	1)	(c)) (P)	(E)	()		G) (H T)		4)
+												-				+	Stair Room Ro
	0.8848	0.1331	0.	7729		0.7511 0.1107	0.6907	0.	6832	0,1168	0.7611		0.7636	0.1351	0.8952		Story10
200		0.6504			5.7600	0.3066 0.4253		6,7600			0.3105	5,7600			0.3360	5,7600	
\vdash		0.4580				0.7287 0.8034					0.7017				1,7048		Story9
0.1000		0.4270		lege.	5,7600	1.4534 0.3787		5.7600			1.4204	57600			1,6991	57600	
		0.9276			2600	0.7377 0.780					0.7146				0.8478	2000	Story8
0	1.7513	0.4432	1.	801	10	1.5072 0.3886	1,4881				1.4789	10			1.7664	10	Story7
00000		0.9354		18/8	97,000	0.7596 0.774		93,000			0.7418	57600	550,500,050		0.8826	27600	
+		0.4504			+	1.5556 0.3968 0.7785 0.7749					0.7653				1.8147	+	Storye
2010	1 8202			***	5,7600			57600				5,7600				5,7600	
+		0.4553				0.7911 0.7763		0.			0.7828			0.4598			Story9
200	1.8207	0.4551	1.	1509	57600	1,6060 0,4118	1.6089	57600	5876	0,4053	1,5912	57600	1.7268	0.4602	1.8207	57600	Story4
2000	0.9189	0.9364	0.	8831	57600	0.7960 0.7789	0.8080	00022	7955	0.7562	0.7918	5,7600		0.9345		57,000	
+		0.4643	_	_		0.7953 0.7825					1.5802				1.8207		Story3
8000		0.9407			57600	1.5904 0.4337		6.7600			1.5059	67,600			1.7993	6,7600	Story2
		0.9366	_		9.9632	0.8021 0.8149				-	0.7267	9,5876		0.9266		27600	
1		0.5103				0.7542 0.8176		1			0.7450				1.7383	-	Story1
					13.5546			8,6185	0.000			13,3595				5,7600	RefPi 1
*		0.3322	_		1424	0.8778 0.2872					0.8669	_			0.9672	8	Plinth Level

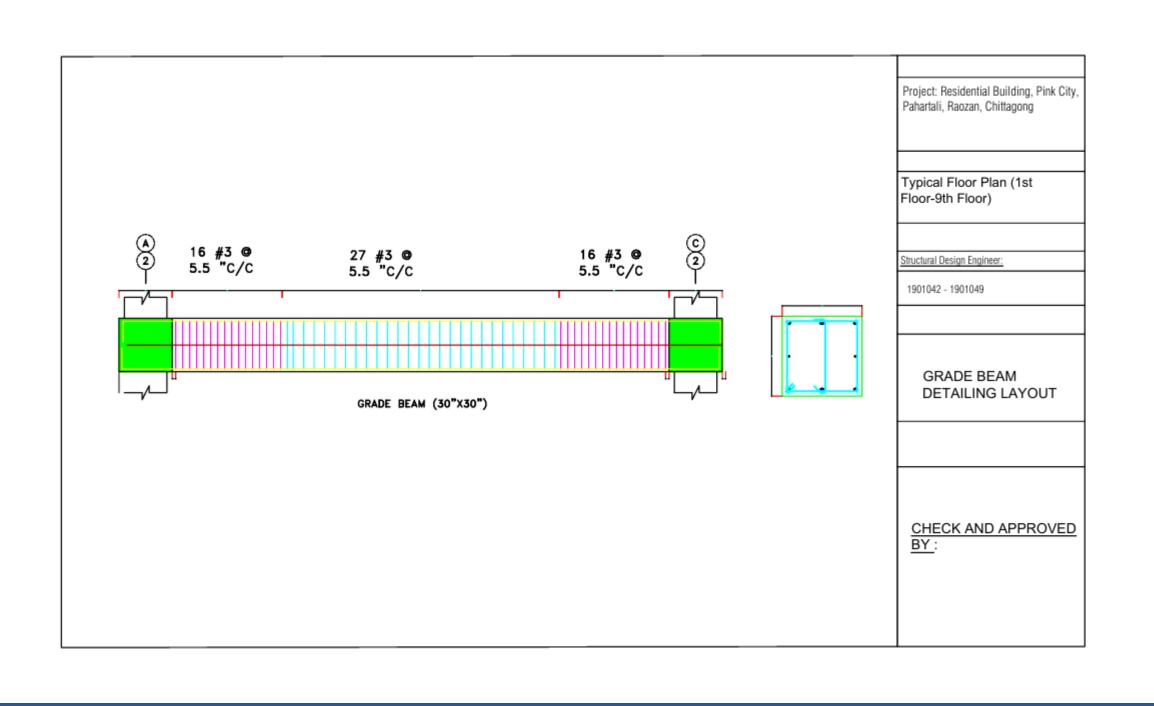
3			>	K		6	{	>	3	4	{ }	3		6	{ }			4	{
7			(T		T)	C		7	, (7		<i>'</i>		7)
+	0.6728					-				-		_		+			0.6620	-	Stair Room Ro
	1.2778		oour Cara			12000	0.9687	0.0871	0.7401		0.7355	0.1060	0.9863	12000		0.3797		6,7600	Story10
	0.2807	0.229	1 0	\$77	7		0.3397	0.4070	0.1644		0.1492	0.4131	0.3560		0.3605	0.2945	0.2714		
-	1,5008	0.978		4.		7,2000	1.4721	0.4272	1.4072	5.7600	1.4314	0.4343	1.4805	7 2000	1.1558	0.0801	1.4522	57,900	
+	0.2981					+			0.7099	-			0.7255	-		0.6579		-	Story9
	1.8207					7,2000			1.4700	6.7600			1.6469	12000		1.2902		6.7900	Story8
	0.5908								0.7393			-	0.8543				0.5616		atoryo
+						7,2000			1.5527	6.7600			1.7500	7.2000				00000	
-	0.7302					+		0.7552		-			0.9132	-		1.4230			Story7
						12000				5,7600				7 2000				5,7500	
-	0.8562					-			1.6197	-			0.9564	-		0.6556			Story6
+						7 2000				57600				12000				6.7900	
-	0.9555					-			0.8444	-			0.9894			1.5988			StoryS
	1.8207					72000			1.7047	57600			1.8207	7 2000		1,6124		6,7900	
1	1.0218	_		-					0.8604			_	0.9986			0.7409			Story4
ł						12000				5.7600				7.2000				6.7900	
	1.8207					-			0.8336	-		-	0.9467	-		0.8107		-	Story3
-						7 2000				67600				7 2000		9.70301B0	11.12.20.00	5,8420	
-	1.8207	0.779				+		0.5242	1.5460	-			1.6687			0.8241	1.8207		Story2
	1.8207					12000			1,4743	5,7600		04.00000	1,4198	12000	550,000	1.2049		92422	40 (00 /* **
-	1.8207		_	-	_			_	0.7212		_	-	0.8221			1.0852			Story1
			5211			12000	000000			6.7600				7,5072	1008.00	1011000100		15,0813	RefPi 1
	22759			-		-			1.0177			_	0.9744	-		0.6365	2.2759		Flinth Level
1	2.2759	4.721	V 0	108	1	7,2000	0.4726	U.9187	0.9812	67600	0.5064	M.9131	0.5358	8,5474	0.6082	U.6365	22799	24.4851	Foundation Le
b	-XX			-		ф				4		-		ф			本	ф	Pouriossion Le

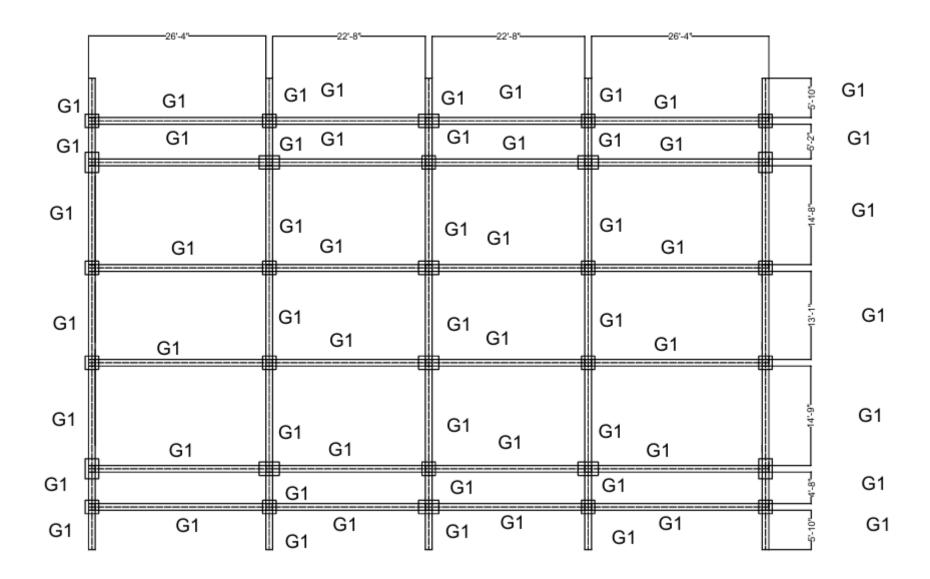


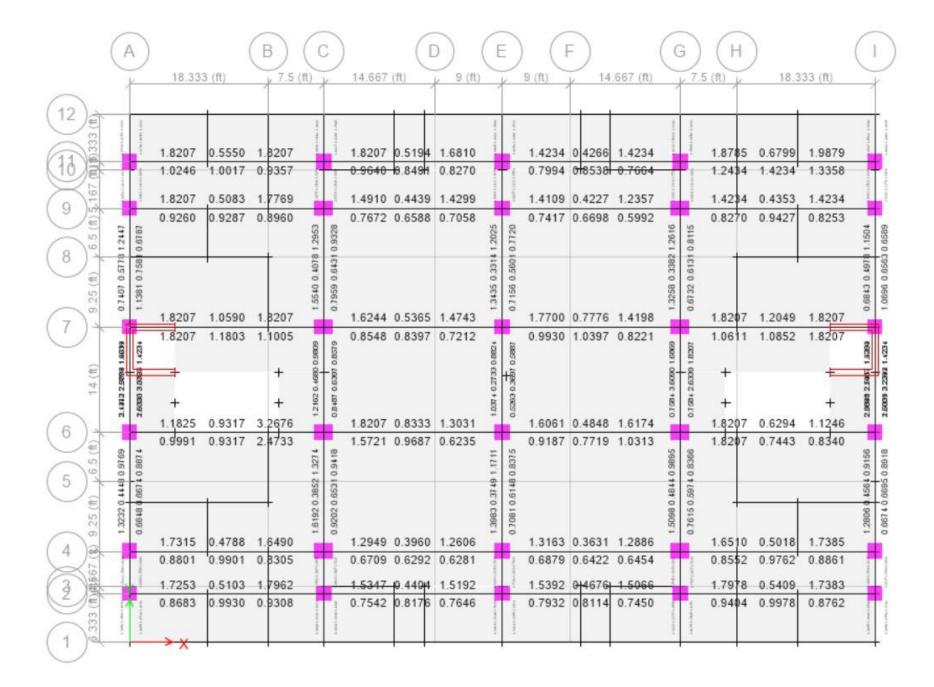


Beam



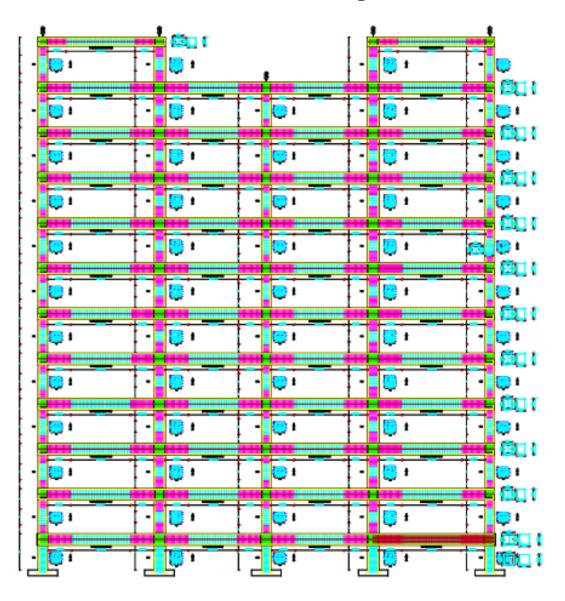




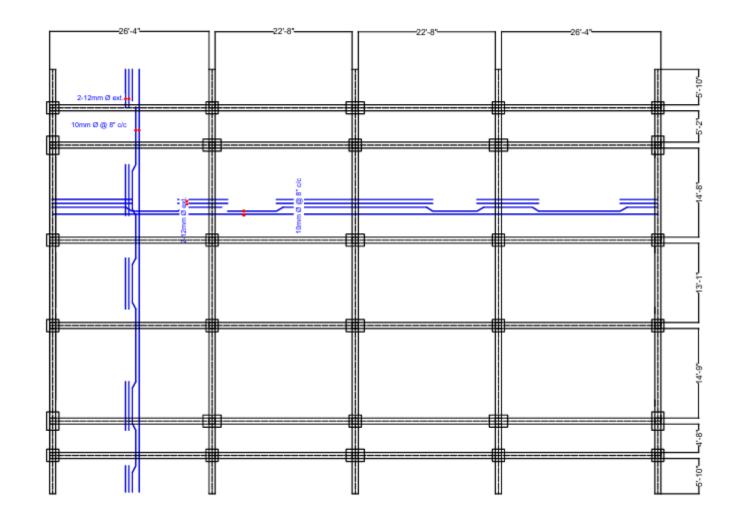


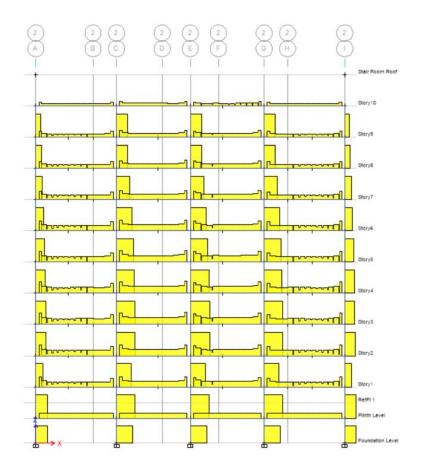
	A)			В	()			/		(F)	0.07 (0)	G) (H)		(
) =	T	18.33	3 (II)	7.5 (11.)		14.667	(11)	9 (ft)			9 (ft)	14	.667 (ft)	1	7.5 (ft)	18.3	333 (ft)	
333 (#)		1.1200	0.3604	1.0628	Total contra	1000	1.0356	0.3427	0.9612	Name and Address of the Owner,		1.0260	0.3307	0.9201	100	1.1323	0.4055	1.1312	
		0.6124	0.5060	0.5793	-	-	0.5726	0.4702	0.5035	100	1	0.5614	0.4694	0.4748	- 1	0.6466	0.5651	0.6333	
167	Ц	1.0727	0.3271	1.0296	1	The state of	0.9921	0.3163	0.9169	decate	The same	1.0203	0.3405	0.8734		1.0621	0.3521	0.9889	
6.5 (#) 5.	1,0985	0.5744	0.4850	0.5557	1545 0.8516	0.8284	0.5406	0.4442	0.4667	0.7648	0.5585	0.5661	0.4882	0.4614	0.5407		0.5187	0.5041	00000
9.25 (#) 6.5 (#)	0.7669 0.6603 1.0985	2.2759	0.8795	1.1599	1.1075 0.4545	0.6076 0.5107	0.9055	0.3644	1.0177	0.9151 0.2650 0.7648	0.4870 0.3955 0.5585	0.9492	0.3477	0.9744	12473 0.5857		0.8703	2.27 <u>59</u>	cupa a duxa a asoc s
_ 0	2.1579	2.2759	0.7210	0.7051	280 0.6230	257 0.8839	0.4726	0.5187	0.5812	305 0.7862	314 0.7316	0.5064	0.5131	0.5358	5451 0.7244		0.6365	2.2759	
41	201713.1399	1.4966	1.4966	+	0.88510.4	0.7526 0.53	1.5205	1.1851	1.6018	0.9254 0.2	0.6270.0.3	1.4786	1.4043	2.1643	0.9324.05	+ 2.2759	1.3041	1.3041	
6.5 (#)		1.4966	1.4966	2.4528	1.1202	009901	1.2969	1.3515	1.2550	0.7986	10.6304	1.5416	1.5669	1.6576	1 2989	2.2759	1.3041	1.3041	00000
(1) 9.25 (ft)	119 0 6969 0	1.0078	0.3235	0.9540	0.87550.4902	0.8608.0.559	0.8750	0.2689	0.8925	0.9917 0.2831	0.5292 0.437	0.8935	0.2672	0.8733	0.7485 0.6492		0.3265	1.0069	0000 + 0501 0 0000 0
67 (1	and the	0.5506	0.4679	0.5169	111	appearing.	0.4626	0.4022	0.4759	A STATE	eChiffins	0.4751	0.4008	0.4630	1000	0.5186	0.4710	0.5523	
		0.9759	0.3322	0.9764		-	0.8778	0.2872	0.8912		1	0.8985	0.2929	0.8669		0.9835	0.3369	0.9672	
6.333 (#		0.5283	0.4874	0.5460	and the case of	A DOUGH STREET, STORY	0.4713	0.4054	0.4878	A A STATE OF THE PARTY OF THE P	1	0.4940	0.4116	0.4621	1000	0.5515	0.4919	0.5214	
July		-> X				+				_	+				+				

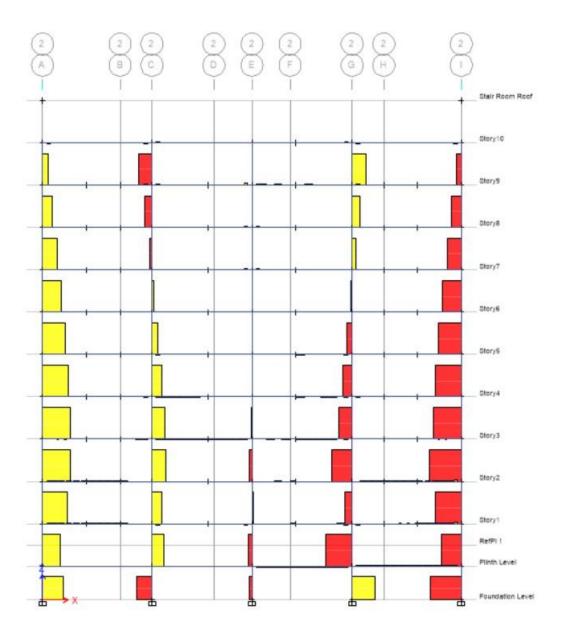
Full Frame Layout

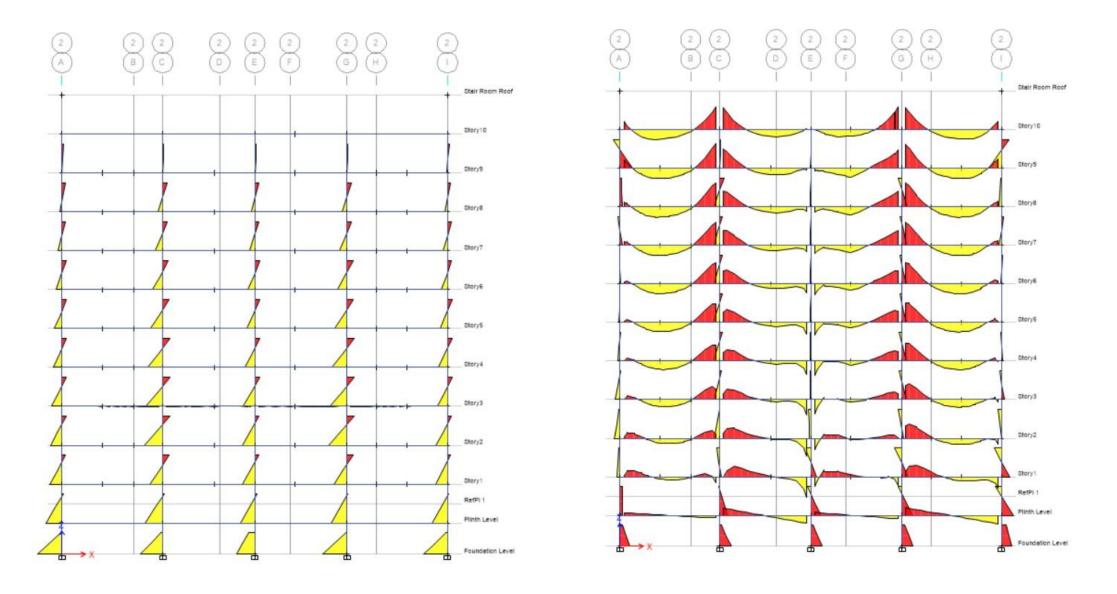


Slab

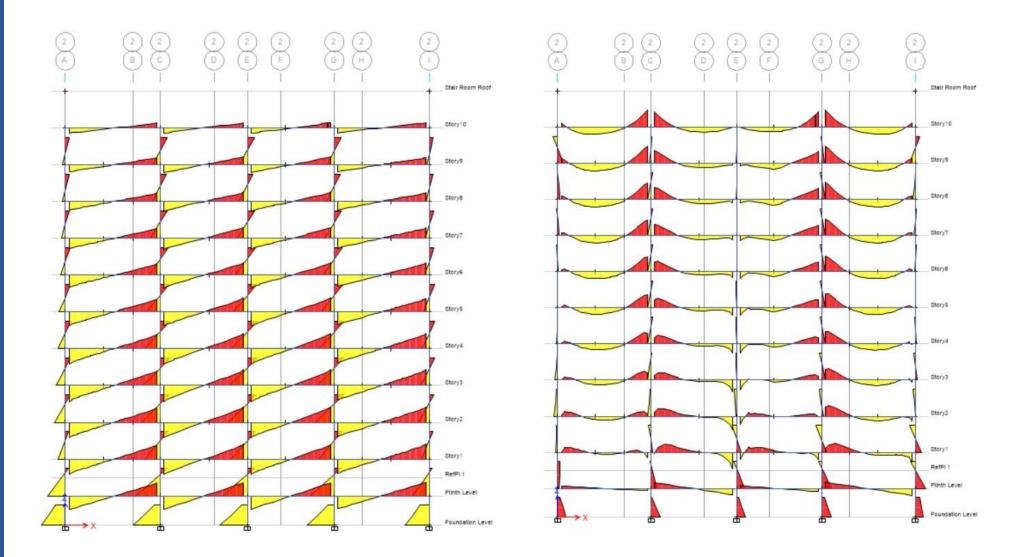


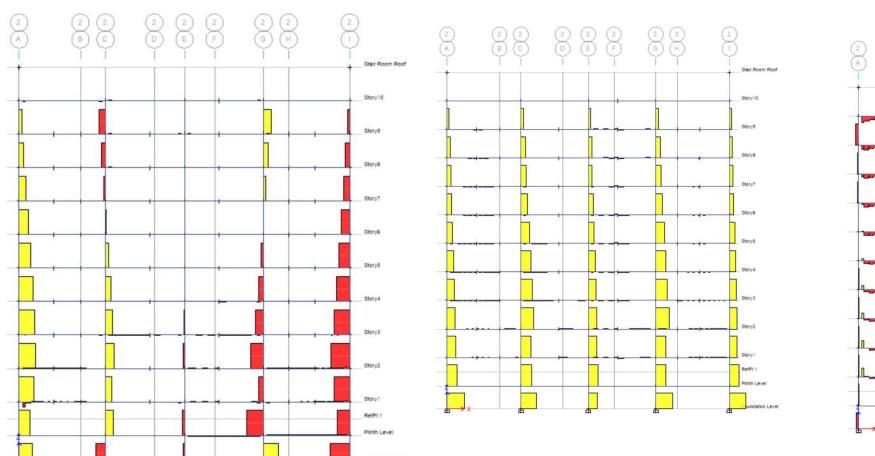


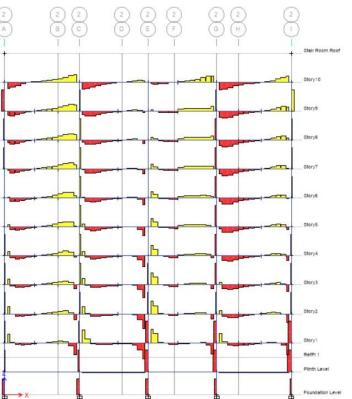




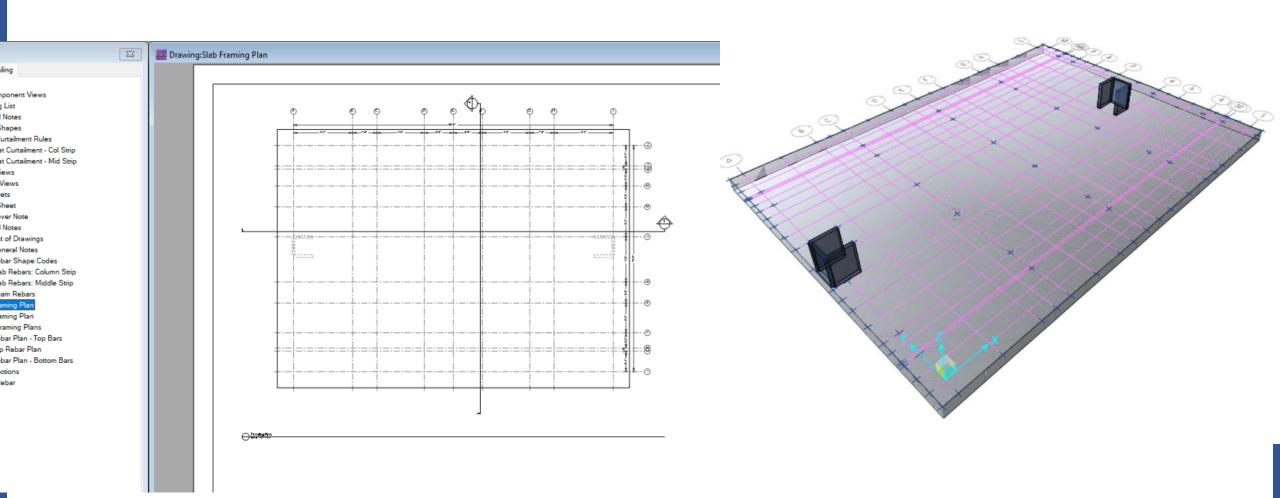






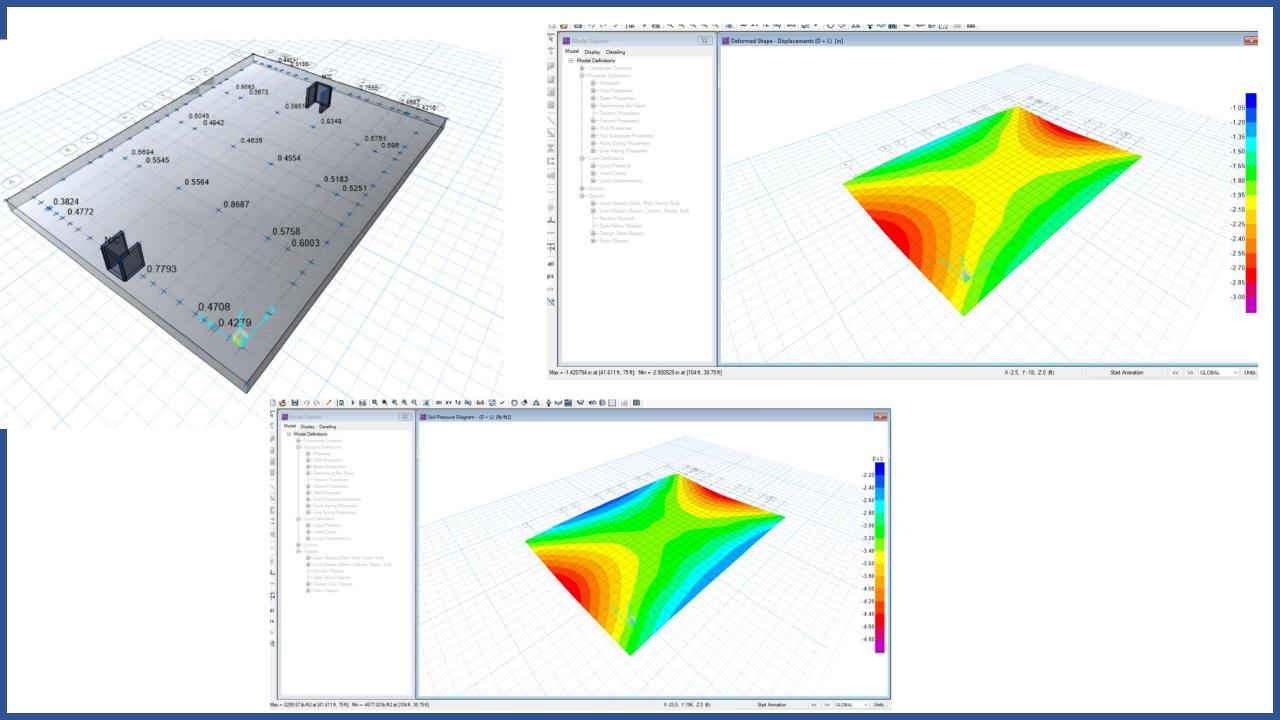


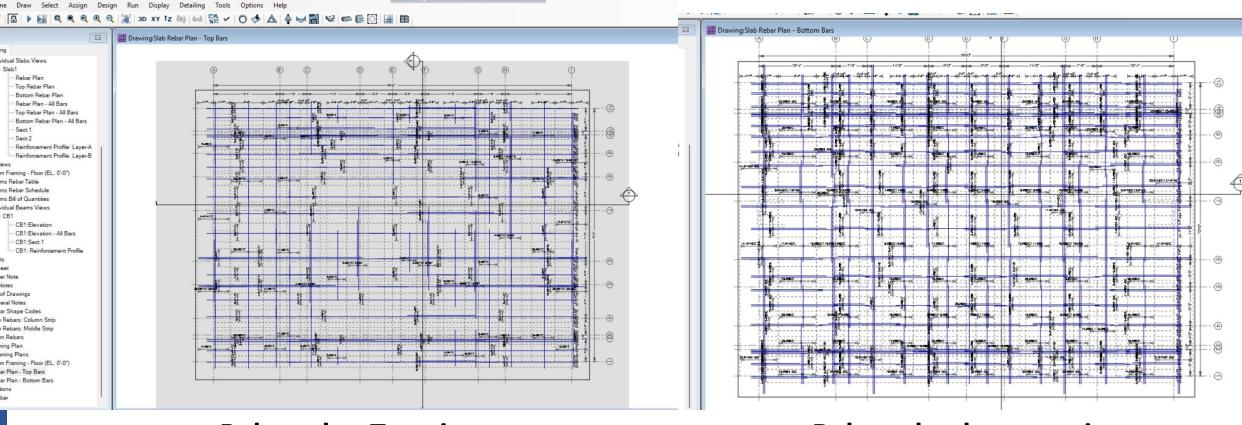
Foundation Design



Borehole: 2 Calculation of site classification: Depth (m) SPT - N d (m) d/SPT -N N 0 0 0 0.333333333 3 1 3 2 0.333333333 1 4 1 0.25 25 0.04 1 5 0.032258065 31 1 6 50 0.02 1 12.61644207 7.5 50 1.5 0.03 9 50 1.5 0.03 10.5 50 1.5 0.03 0.03 12 50 1.5 13.5 50 1.5 0.03 15 0.03 50 1.5 Sum 15 1.188924731

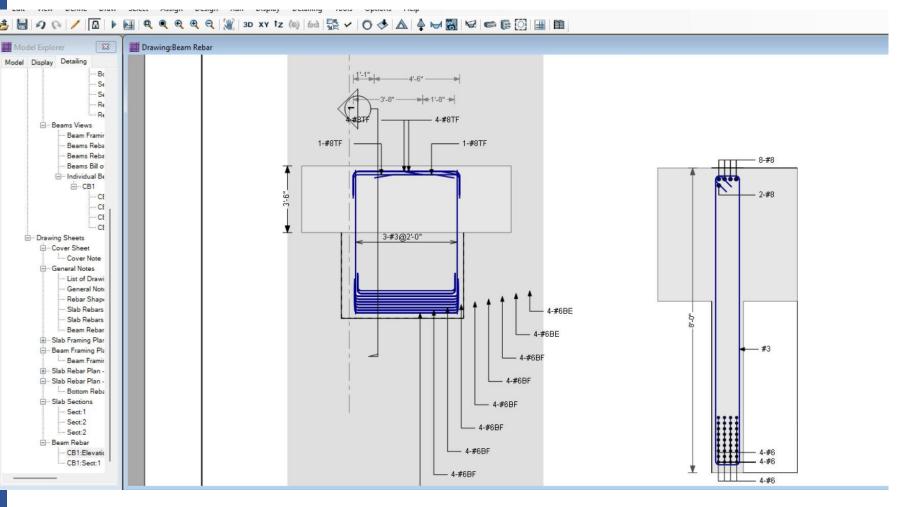
Site class: SD





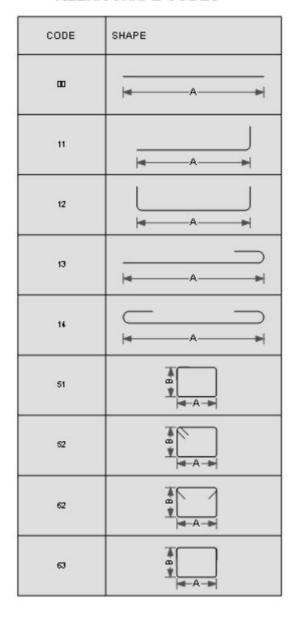
Reber plan Top view

Reber plan bottom view



Beam Elevation

REBAR SHAPE CODES





Cost Analysis

The Estimate & BOQ of Proposed 10-Storied Residential Building

i iooi riica.			
	Ground Floor=	7000.00	Sft
	1st Floor=	7000.00	Sft
	2nd Floor=	7000.00	Sft
	3rd Floor=	7000.00	Sft
	4th Floor=	7000.00	Sft
	5th Floor=	7000.00	Sft
	CHI FILL	7000.00	

7th Floor= 7000.00 Sft 8th Floor= 7000.00 Sft 9th Floor= 7000.00 Sft Total= 7000.00 Sft

Foundation Work:-

SI.No	Description	Quantity	Rate	Unit	Amount, TK
1	Earth cutting	35000.00	70.00	Cft	2450000.00
2	Earth & Sand Filling	2500.00	20.00		50000.00
-	Earth & Sand Fining	2500.00	20.00		30000.00
а	Cement	1000.00	520.00	bag	520000.00
b	Sand (Local)	600.00	50.00	Cft	30000.00
с	Sand (Sylhet)	500.00	100.00	Cft	50000.00
d	Stone Chips	3000.00	180.00	Cft	540000.00
e	Rebar	123740	90.00	Kg	11136600.00
		Total			12276600.00
3	Septic Tank				
	10" Brick wall				
	RCC Base & Slab				
	Casting, 1:2:4(Brick Chip)				
a	Cement	30.00	520.00	bag	15600.00
b	Sand (Local)	150.00	50.00	Cft	7500.00
С	Brick	1500.00	10.00	No	15000.00
		Total			38100.00
4	Under Ground Water				
	Reservoir, Base & Rcc Wall				
	Stone Casting, 1:1.5:3				
	Slab- Brick Chips				
	Casting,1:2:4				
a	Cement	250.00	520.00	bag	130000.00
b	Sand (Local)	220.00	50.00	Cft	11000.00
С	Sand (Sylhet)	200.00	100.00	Cft	20000.00
d	Stone Chips	1200.00	180.00	Cft	216000.00
e	Brick for Soling& outer wall	7000.00	10.00	No	70000.00
f	Rebar	3000	90.00	Kg	270000.00
g					0.00
		Total			717000.00
	Labour Cost				500000.00
	Foundation Total				16031700.00

Cost For Project:-				
Description				Amount TK.
Foundation				16031700.00
Ground Flooor				3868080.00
1st to 9th floor	7148400.00		9	64335600.00
Lift, Generator				4000000.00
Net Total				88235380.00
5% Over Head Cost				4411769.00
Gross Total				92647149.00
	Description Foundation Ground Flooor 1st to 9th floor Lift, Generator Net Total 5% Over Head Cost	Description Foundation Ground Flooor 1st to 9th floor 7148400.00 Lift, Generator Net Total 5% Over Head Cost	Description Foundation Ground Flooor 1st to 9th floor Lift, Generator Net Total 5% Over Head Cost	Description Foundation Ground Flooor 1st to 9th floor Lift, Generator Net Total 5% Over Head Cost

Tota	al Cost For Project:-			
SL	Description			Amount TK.
1	Foundation			15144520.00
3	Ground Flooor			3788700.00
4	1st to 9th floor	6967744.00	9	62709696.00
5	Lift, Generator			400000.00
6	Net Total			85642916.00
7	5% Over Head Cost			4282145.80
	Gross Total			89925061.80

